

Steel and composite bridges in Germany State of the Art

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- Typical composite road bridges with open sections and box girders
- Composite box girders with wide cantilevering concrete decks
- Composite bowstring arches
- Composite trusses
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- Cable stayed bridges
- Canal bridges

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Advantages of composite bridges

- **Very slender and aesthetic bridges** due to the optimal combination of high tensile strength of structural steel and the high compressive strength of concrete
- **High durability** of normal reinforced concrete decks due to restrictive crack width limitation.
- In comparison with steel bridges composite bridges have a **better behaviour with regard to freezing in winter.**
- Because the **low dead weight** of the composite bridges deck is, composite bridges have advantages with regard to the foundation and settlements of supports.
- Due to **innovative erection methods** composite bridges are often used for bridges over passing existent railways or highways without any restrictions for the traffic. Where existing freeways with two lanes are widened the short erection time of composite bridges avoids longer restrictions for traffic.

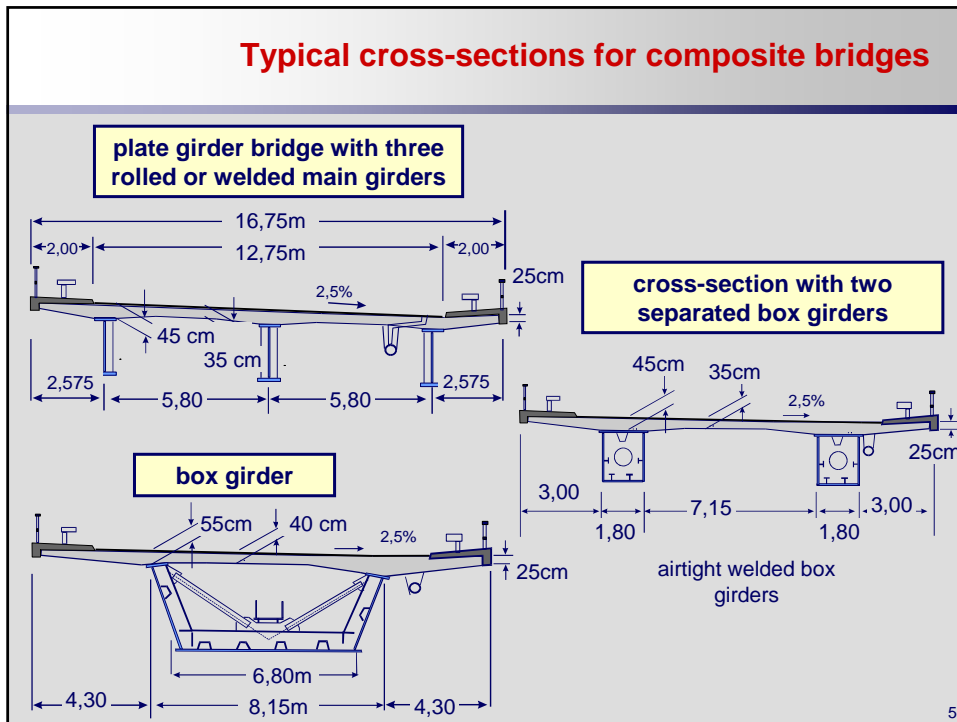
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Composite Bridges with open and closed cross-sections



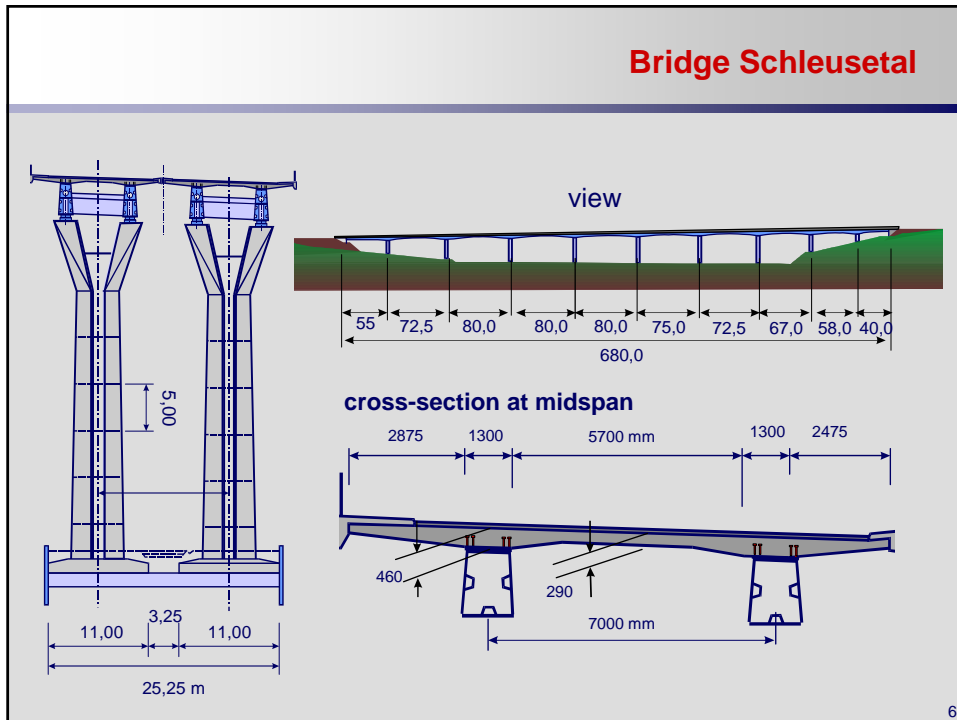
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Typical cross-sections for composite bridges



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Bridge Schleusetal



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Bridge Schleusetal



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Bridge Schleusetal

transportation of steel girders on site



erection of steel girders by cranes

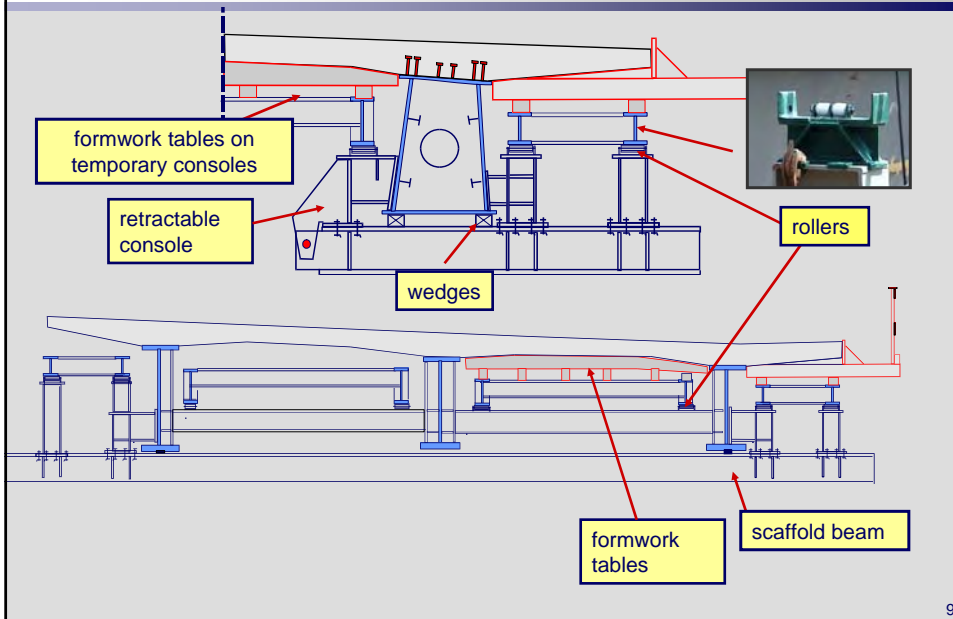


casting of the concrete deck by moveable formwork



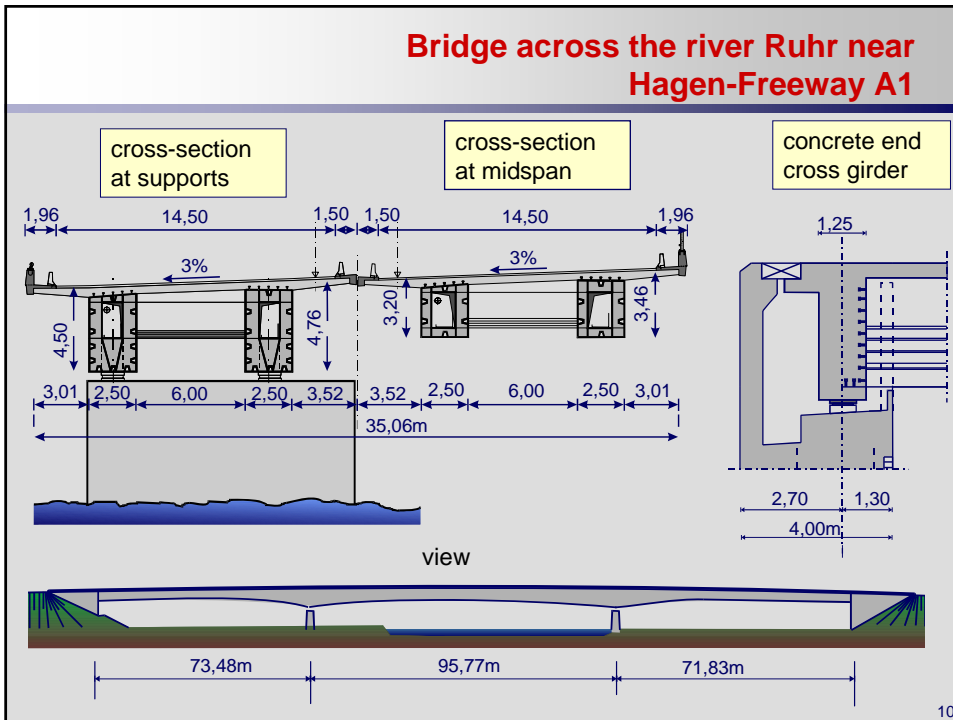
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Examples for formwork systems



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Bridge across the river Ruhr near Hagen-Freeway A1



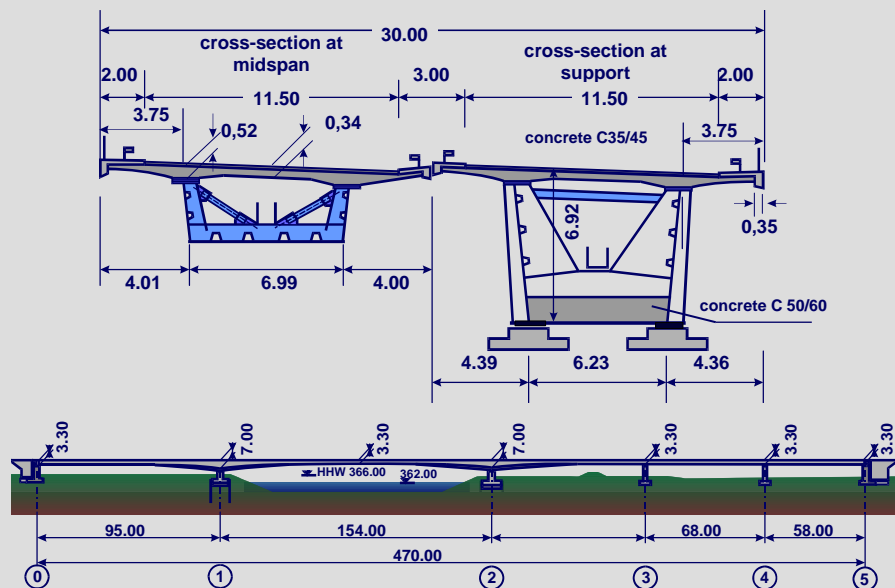
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Bridge across the river Ruhr near Hagen-Freeway A1



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Bridge Neuötting -Double composite box girder



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Double composite bridge Neuötting



box girder with double composite action at internal supports



composite bottom flange

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Box girders with corrugated webs

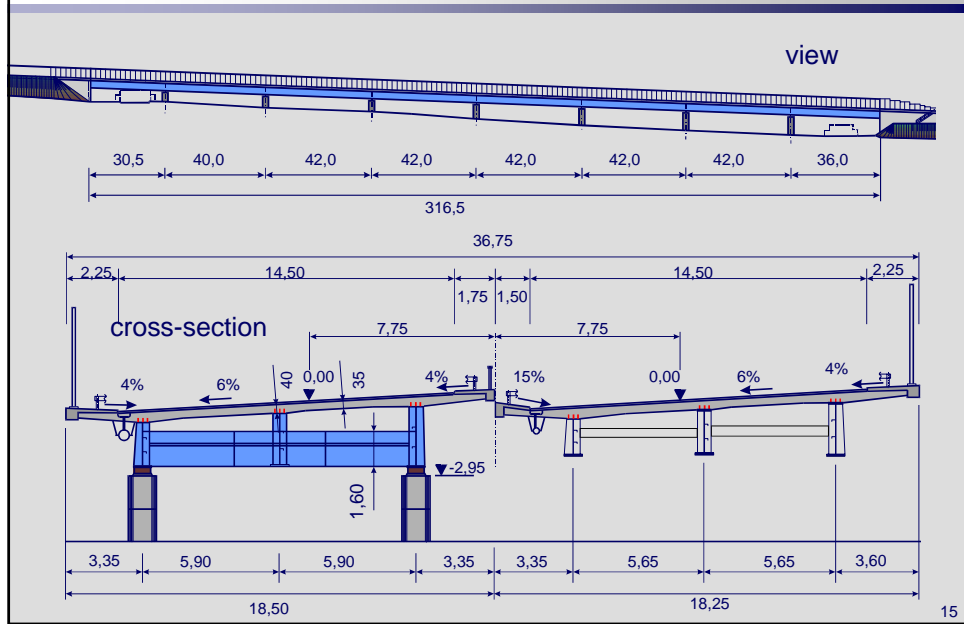


Bridge
Altwipfergrund



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Bridge Langerfeld – Freeway A1

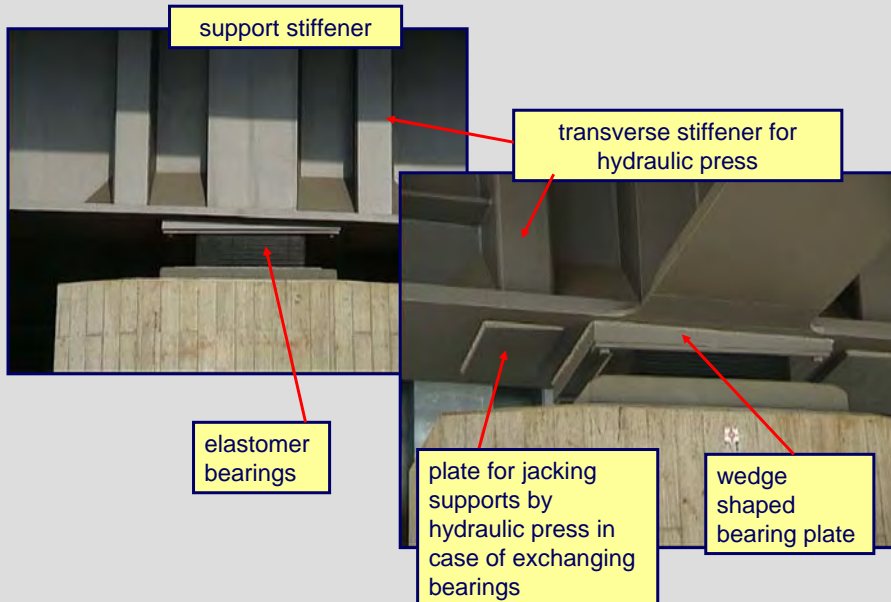


Langerfelder Bridge – Highway A1



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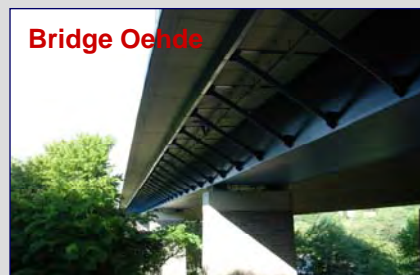
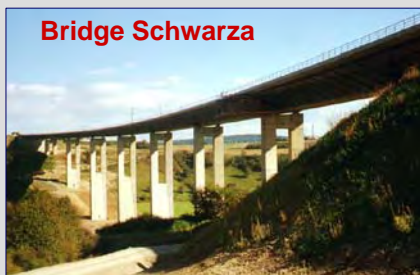
**Langerfelder Bridge – Highway A1
Detailing at internal supports**



**Composite box girders with wide cantilevering
concrete decks**

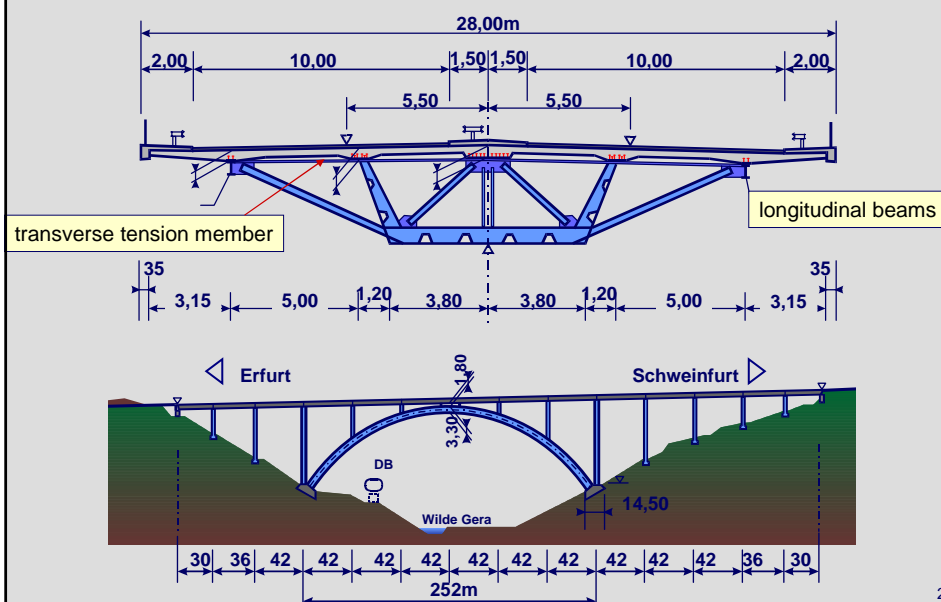


Composite box girders with wide concrete decks



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Bridge Wilde Gera



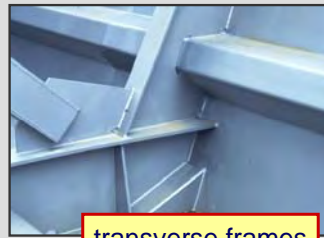
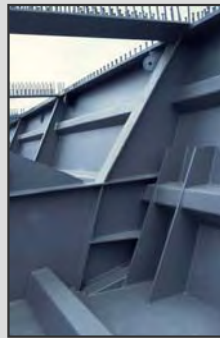
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Bridge Wilde Gera



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Bridge Wilde Gera – Highway A71

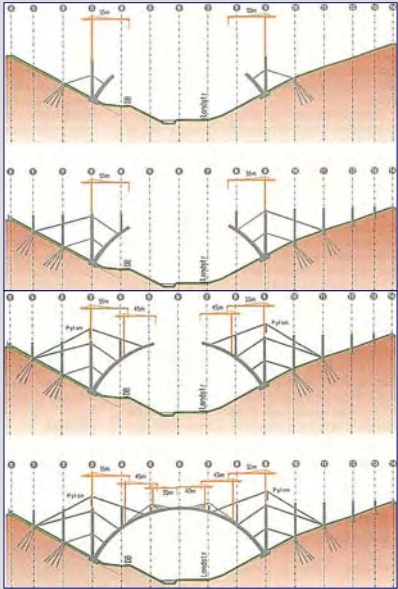


transverse steel
tension members

transverse frames

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Bridge Wilde Gera – Highway A71



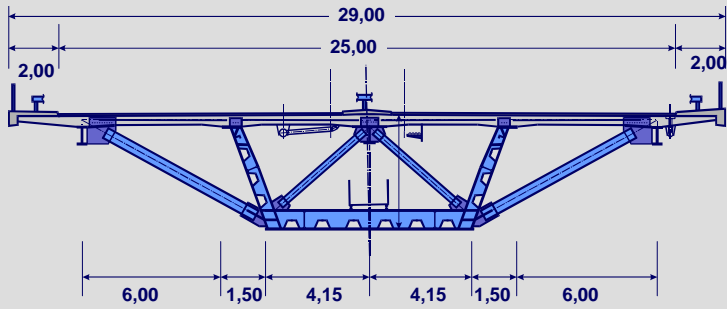
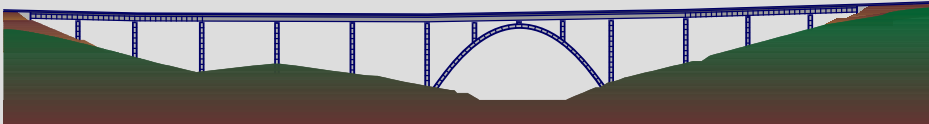
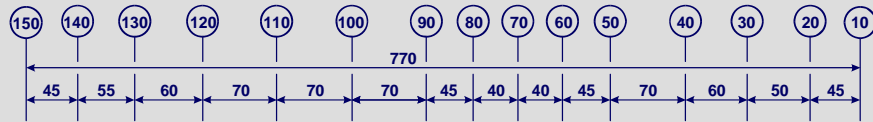
cantilever erection of the arch

Bridge Wilde Gera – Highway A71



Bridge girder during launching

Bridge Albrechtsgraben - Highway A71



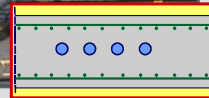
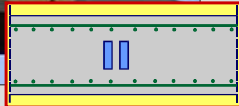
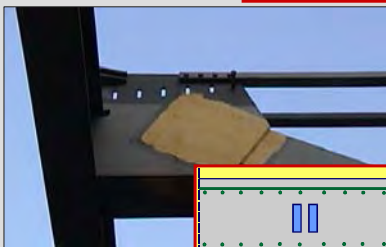
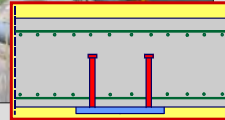
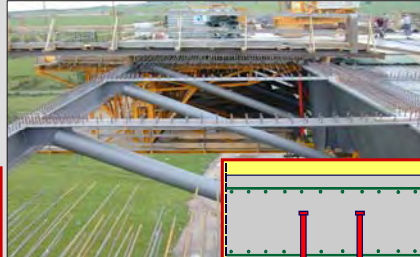
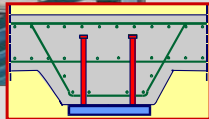
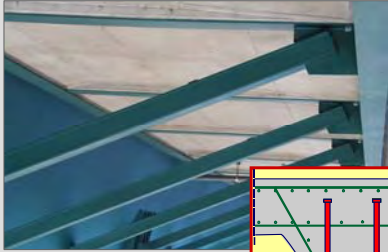
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Bridge Albrechtsgraben - Highway A71



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Different detailing of transverse tension members



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Joints between the longitudinal girders/box girders and the transverse tension members



Bridge
Albrechtsgraben

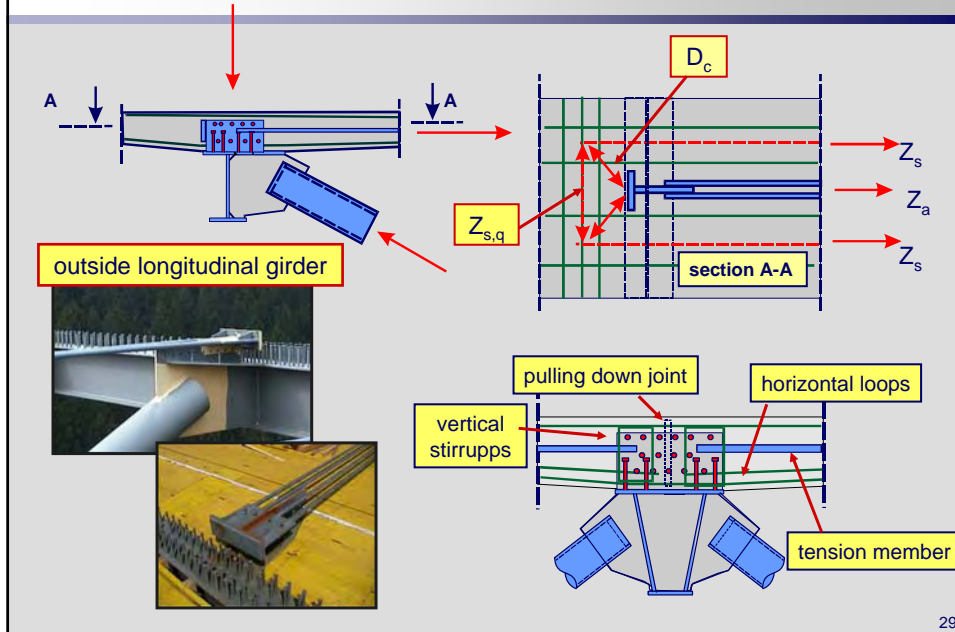


Bridge
Schwarza



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Detailing of the transverse tension members



Assessment of different solutions for transverse tension members

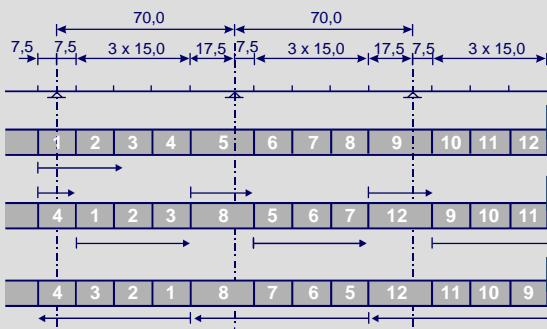
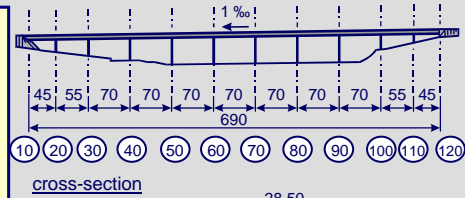
	geometry of the concrete slab	internal forces in the tension member	connections
	a constant depth of the slab over the total width of the bridge deck is required, uni-axial load transfer in transverse direction	high stresses due to local concentrated wheel loads (Fatigue resistance)	economical simple connections with the top flanges of the box girder and the secondary longitudinal girders
	haunched concrete slab, expensive formwork, bi-axial load transfer in transverse and longitudinal direction	higher stresses due to local loads. Resistance of the tension member to normal forces, bending moments and vertical shear should be considered	
	slabs with constant depth and haunches in longitudinal direction are possible, uni-axial load transfer in transverse direction	no significant stresses in the steel tension member due to local loads	complicate connection caused by the eccentricity between the tension member and the top flanges of the box girder

Sequense of casting of concrete

Method I : continuous in one direction

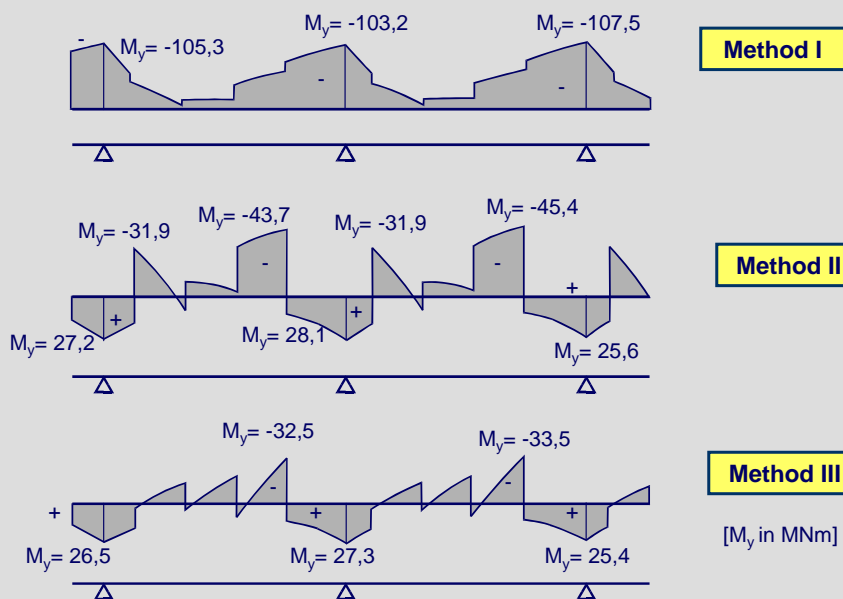
Method II: continuous in one direction but regions at internal supports are casted in a second step after the concrete in mid-span regions is effective

Method III: span-wise in reverse direction



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bending moments acting on the composite section due to dead weight of concrete

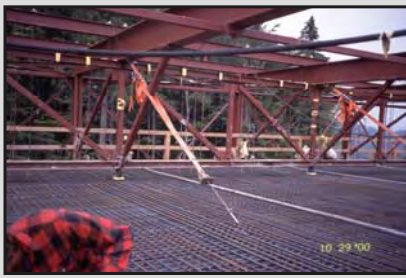
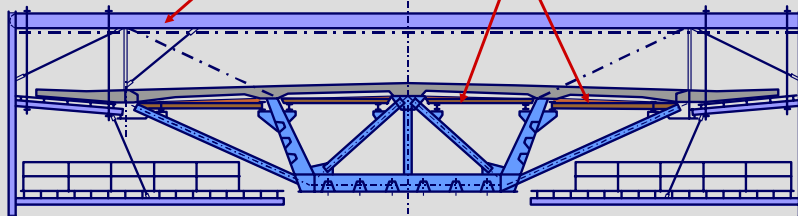


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Formwork carriage running on the top of the deck

formwork carriage for cantilevers

formwork tables on temporary consoles



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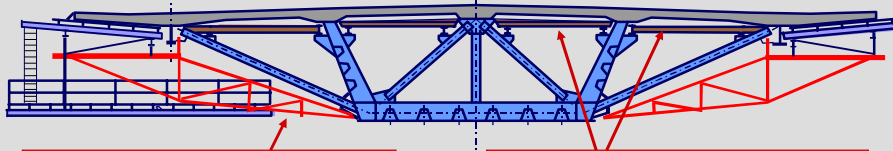
Formwork carriage running on the top of the deck



Bridge Wilde Gera

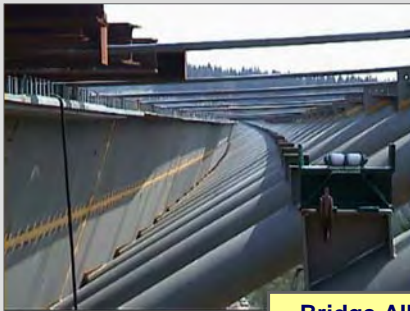
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Formwork underneath the concrete deck



formwork carriage for outer cantilevers

formwork tables on temporary consoles



Bridge Albrechtsgraben

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Formwork underneath the concrete deck (system Kirchner)



Bridge Schwarza

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Exchange of concrete decks

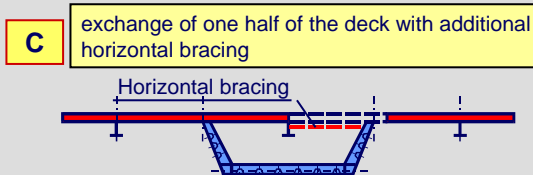
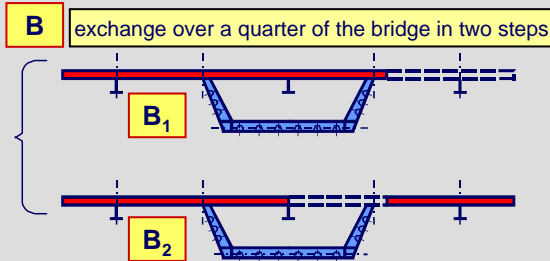
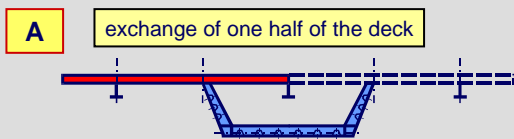


Normally it is a general requirement in Germany to have separate bridges for each traffic direction in order to be able to divert the full traffic on the remaining bridge in case of major maintenance work on the other. The concrete deck is the most vulnerable part of a bridge section. With regard to the expected intensive increase of road traffic and local wheel loads in future, the concrete deck must be regarded as a wearing part in contrast to the steel structure with implication of different lifetimes of the concrete deck and the steel structure.

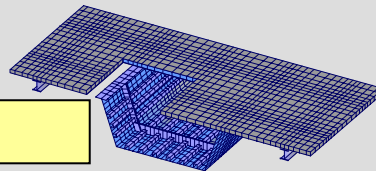
In case of one main composite box girder for both traffic directions the flow of traffic has to be maintained in both directions just on one half of the bridge during a future replacement of the bridge deck. For this procedure the bridge deck will be partially cut out with high-pressure water method and will be replaced by a new bridge deck. During this procedure significant additional stresses result in the superstructure, which have to be considered during design and construction.

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Different possibilities for the exchange of the concrete slab

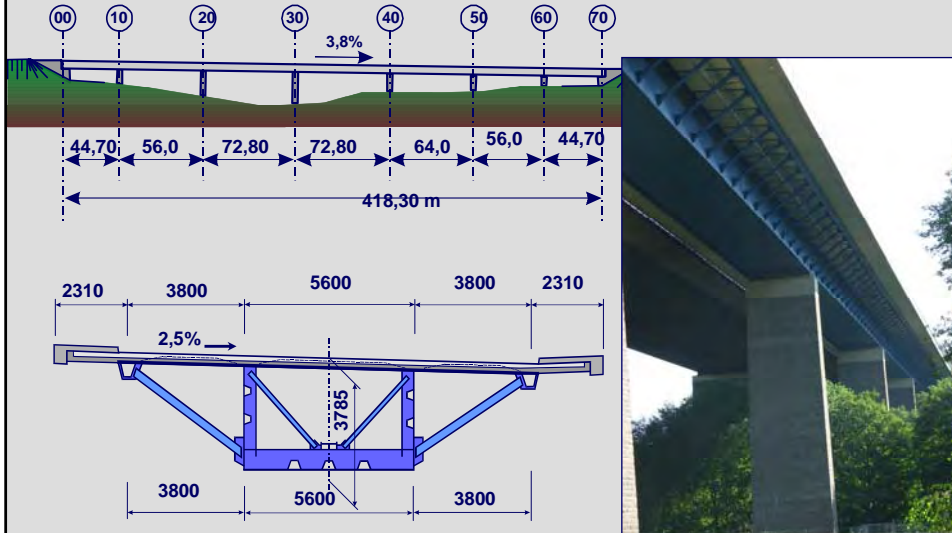


The width of the span is the most important factor for choosing the different methods. Method A can only be used for spans up to 50 m. Methods B or C are possible for the exchange of the concrete deck of bridges with larger spans of up to 100 m.



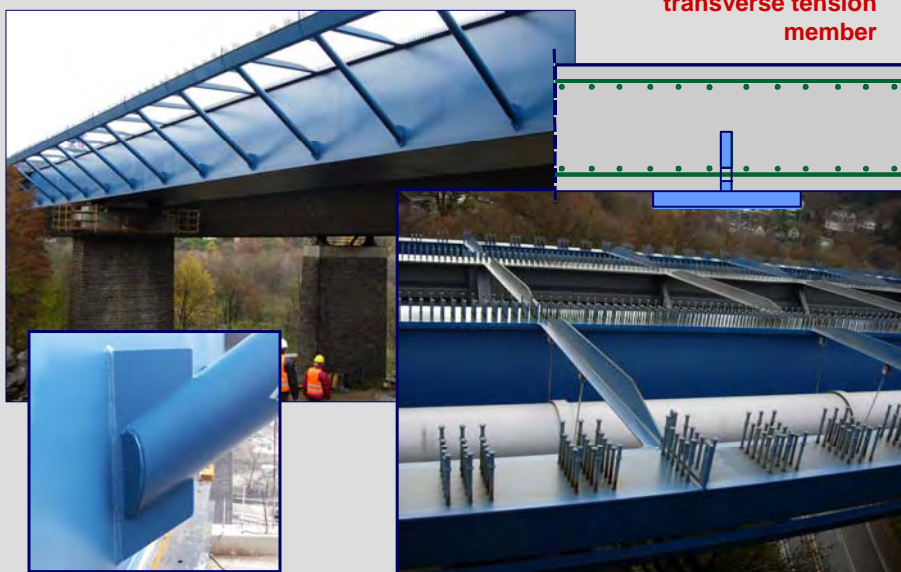
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Bridge Oehde – Freeway A1



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Bridge Oehde – Freeway A1



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Bridge Oehde – Freeway A1

P1 P3 P2 P6 P4 P5 P9 P7 P8 P11 P10 P13 P12 P15 P14



launching of the bridge with partially prefabricated concrete elements



concrete deck with partially prefabricated concrete elements



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Composite bowstring arches



Saalebridge Besedau

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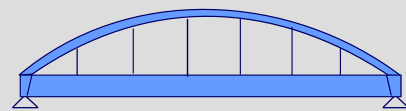
Composite bow string arches



Composite bow string arches with concrete decks are an often used system where the construction depth is limited e.g. for bridges over canals and rivers. The reinforced concrete deck is connected with the steel structure by horizontal bracings at the end of the bridge and is acting as a tension member in the main system.

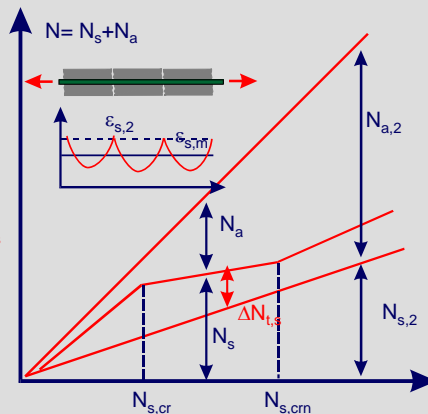
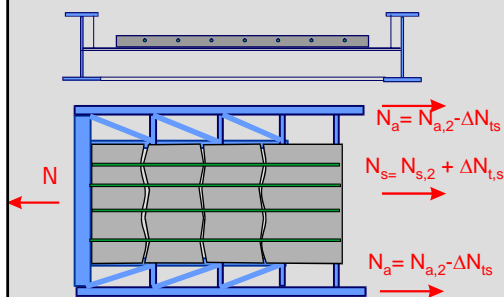
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Normal forces in the concrete tension member



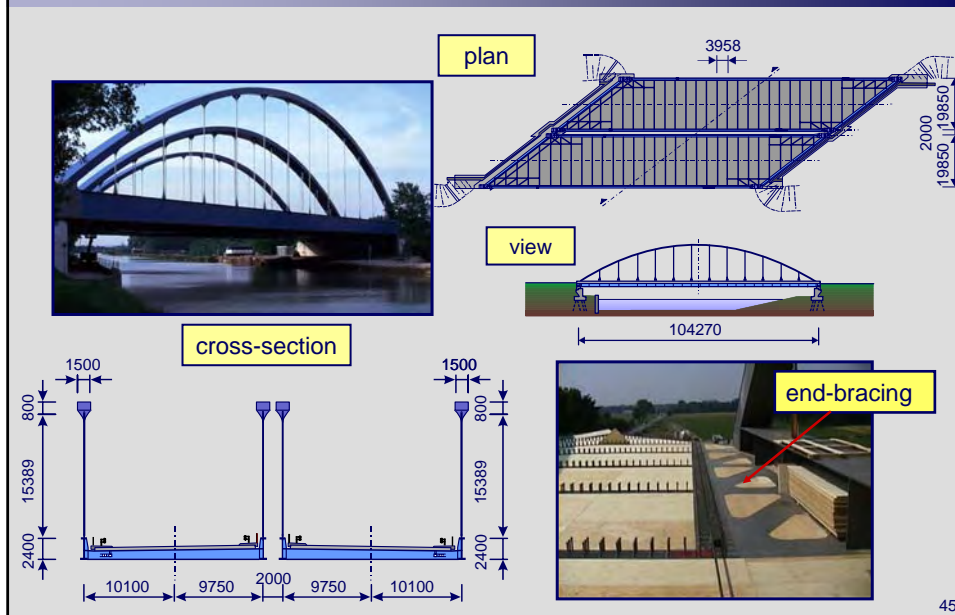
The concrete member acts as a tension member in the main system.

For the normal forces in the concrete member the effects of tension stiffening of concrete between cracks should be considered.



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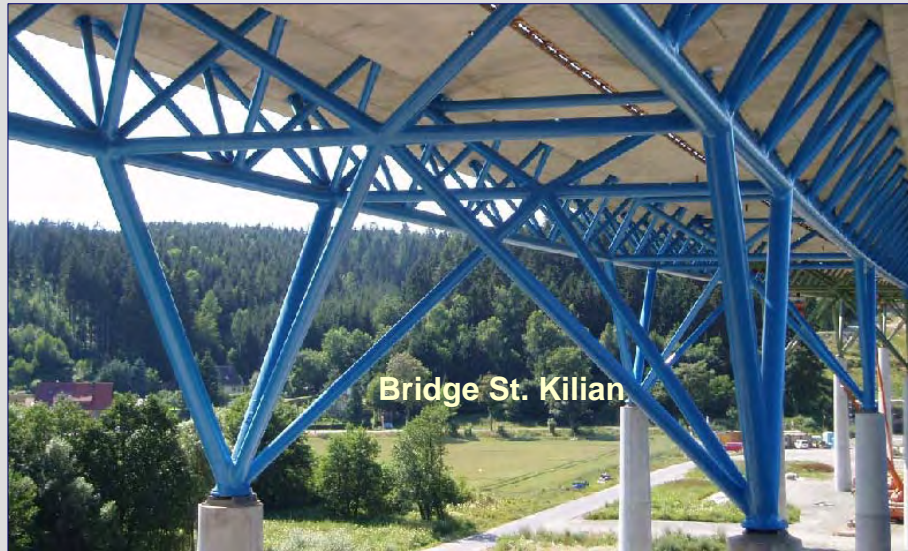
Bridge Ladbergen near Ladbergen– Highway A1



Detailing of composite bowstring arches

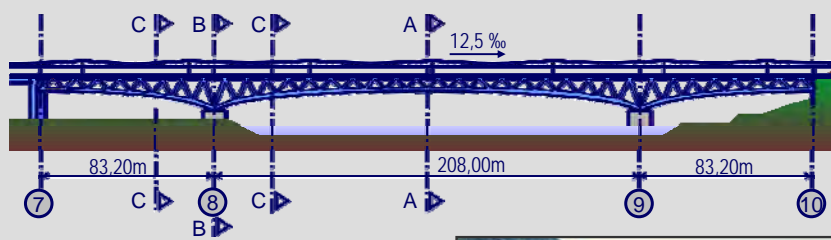


Composite trusses

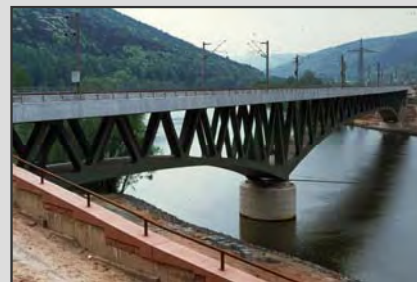


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Composite truss bridge with double composite action – Bridge Nantenbach across the river Main

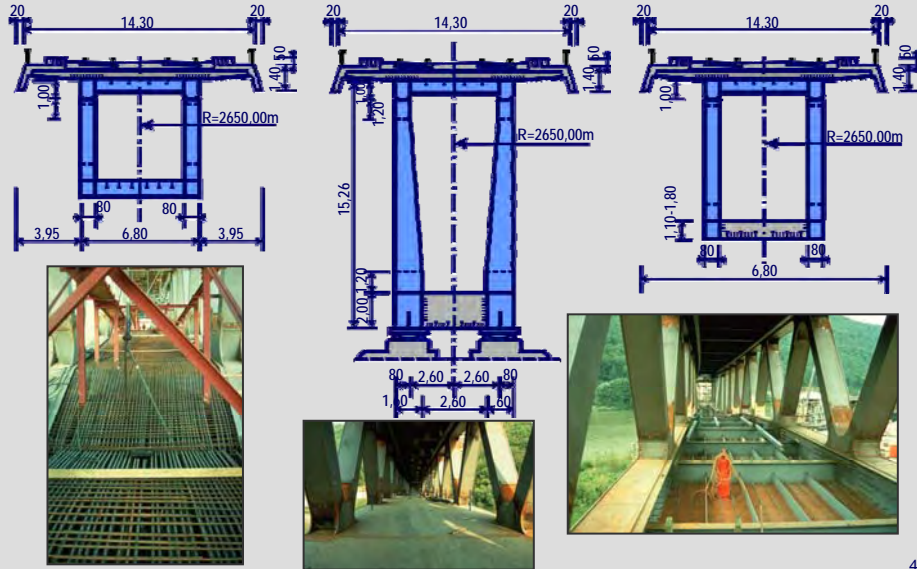


view



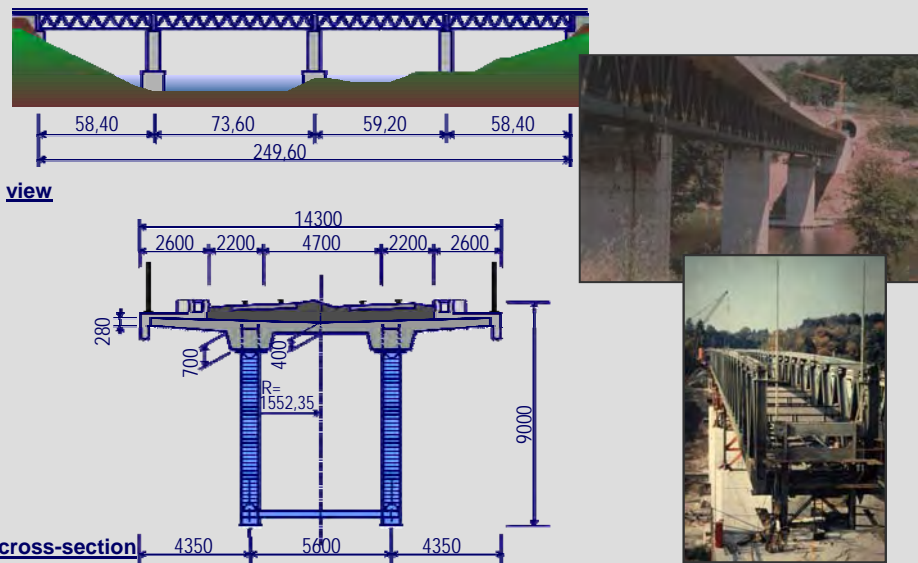
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Composite truss bridge with double composite action – Bridge Nantenbach across the river Main



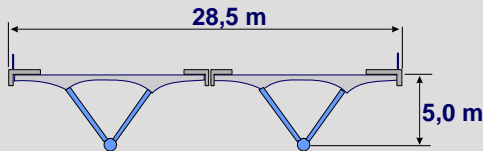
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Composite truss bridge Kragenhofer



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Bridge St. Kilian



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Bridge St. Kilian



steel trusses with hollow sections and cast iron nodes



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Composite bridges for small and medium spans



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Composite bridges with rolled sections for small and medium spans



Advantages

- short construction time
- simple erection method because of no steelwork on site.
- steelwork only in the shop
- small total depth of the composite section
- concrete slab without any pre-stressing

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Longitudinal reinforcement at internal supports

concentrated shear connectors at the end of the girder

l_b -anchorage length of the reinforcement

bottom layer of reinforcement
 $A_{s,min} = \text{Ø}16 \text{ a}=10 \text{ cm}$

- The longitudinal reinforcement at internal supports should be placed over a length not smaller than $L = 0,15 L_{St}$ where L_{St} is larger span length adjacent to the support considered.
- at the end of the steel girder the number of shear connectors should be increased due to local introduction of the tensile force in the reinforcement in the composite section.
- In case of sagging bending moments at internal supports due to temperature effects, traffic loads and settlements of supports for the tensile forces in the bottom flange a connection by steel plates or studs in combination with loops is required.

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Detailing of transverse supporting beams in concrete

$b_{min} = 80 \text{ cm}$ (indirect support)
 $b_{min} = 60 \text{ cm}$ (direct support)

$b > b_{mig}$

$b > 90 \text{ cm}$

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Detailing of partially prefabricated concrete elements

$\geq C_{nom} = 4,5\text{cm}$

h_c

mortar

4,0

2,5

1,0

21

seal

shear reinforcement

$\geq 3\text{cm}$

$\geq 2,5\text{ cm}$

the depth of the concrete above the prefabricated elements h_c should be not less than 20 cm within the traffic lanes. in other regions h_c should not be less than 15 cm.

Elastomeric support strips with a thickness of 2 cm and a width of 3 cm. The minimum value of pressing should be 3-5 mm and the maximum value should not exceed 10 mm.

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Casting of concrete

in case of smaller span length the transverse supporting beams and the concrete slab should be casted in one operation.

in case of larger span length or for bridges with a big total length at first the midspan regions and subsequently the internal supports and the transverse supporting beams should be casted.

0,15 L

L

casting of mid-span regions

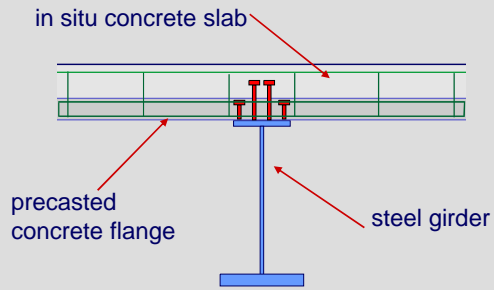
casting of supporting beams and the slab at internal support

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VFT-Bridges with welded or rolled I-sections



steel girder in the shop



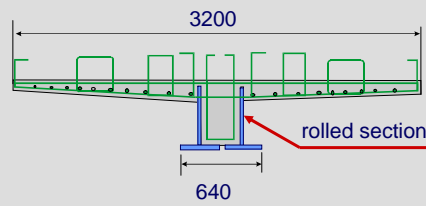
installation by crane



transport on site

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VFT-Filler beams



shear connection

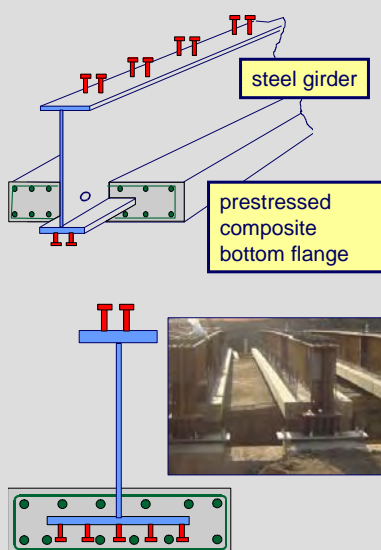
60

VFT-Filler beams



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Preflex - Beams



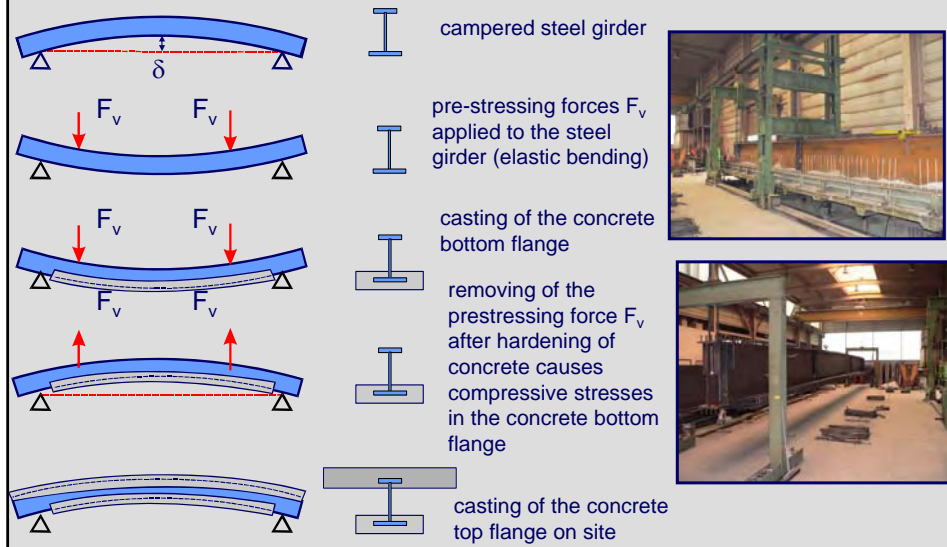
The Preflex-Beam is a girder with a pre-stressed composite bottom flange where the pre-stressing is applied by elastic bending of the steel girder.

The pre-stress causes a higher bending resistance and a high flexural stiffness. Therefore the deflections under serviceability conditions very small. This type of beam is often used for railway and road bridges where the available construction depth is highly restricted. Ratios of span to structural depth up to 45 are possible for road bridges.



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Preflex Girders – pre-stressing of the concrete bottom flange



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Composite bridge with Preflex-Girders



Hermann-Lieberum Bridge in Leipzig

continuous beam, span 33 m and construction depth 130 cm



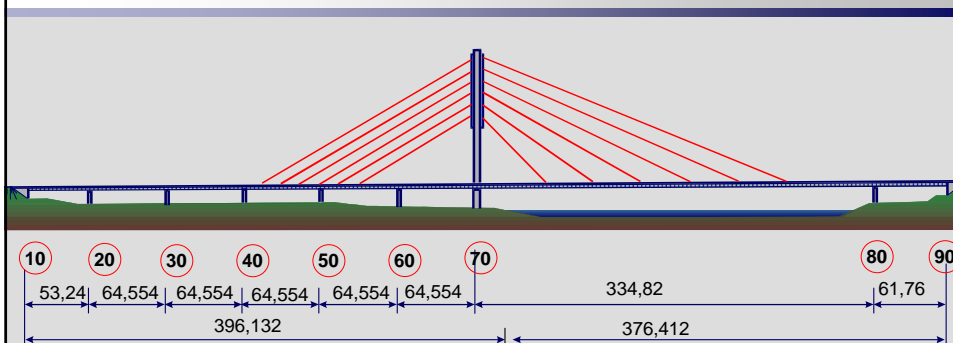
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Cable stayed bridges



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cable stayed bridge Wesel across the river Rhein

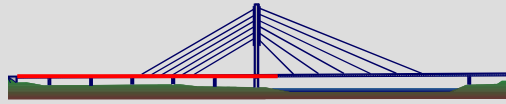


10 20 30 40 50 60 70 80 90
53,24 | 64,554 | 64,554 | 64,554 | 64,554 | 64,554 | 334,82 | 61,76
396,132 376,412

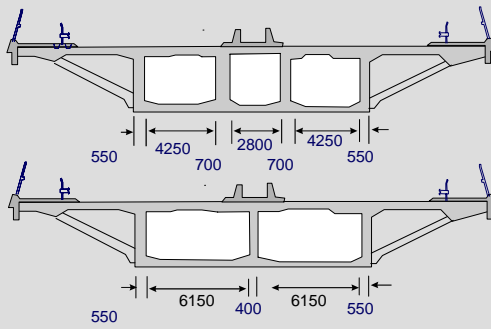


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Cable stayed bridge Wesel across the river Rhein

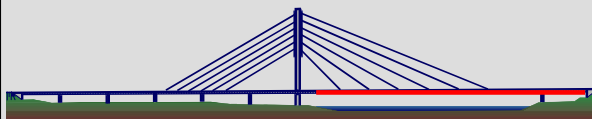


cross-section axes 10 - 70

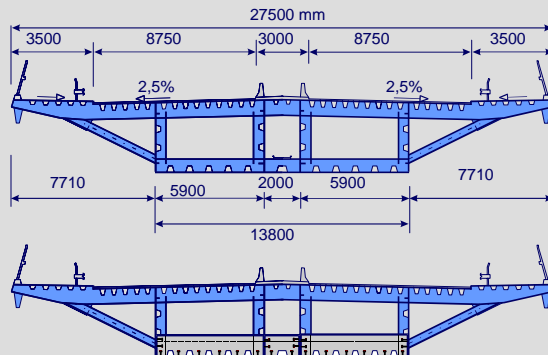


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Cable stayed bridge Wesel across the river Rhein



cross-section axes 70 - 90



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Joint between the steel and the concrete section – concrete end cross girder



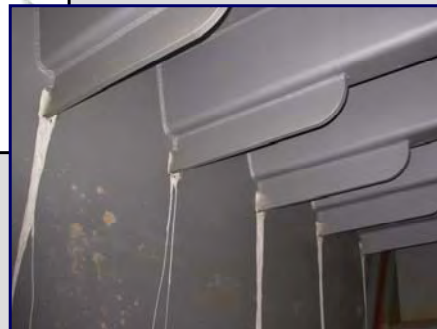
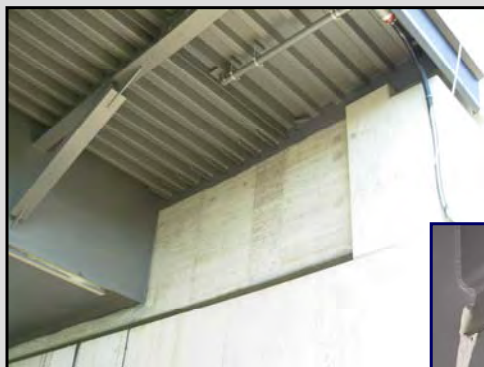
joint with overlapping steel plates and headed studs



concrete end cross girder

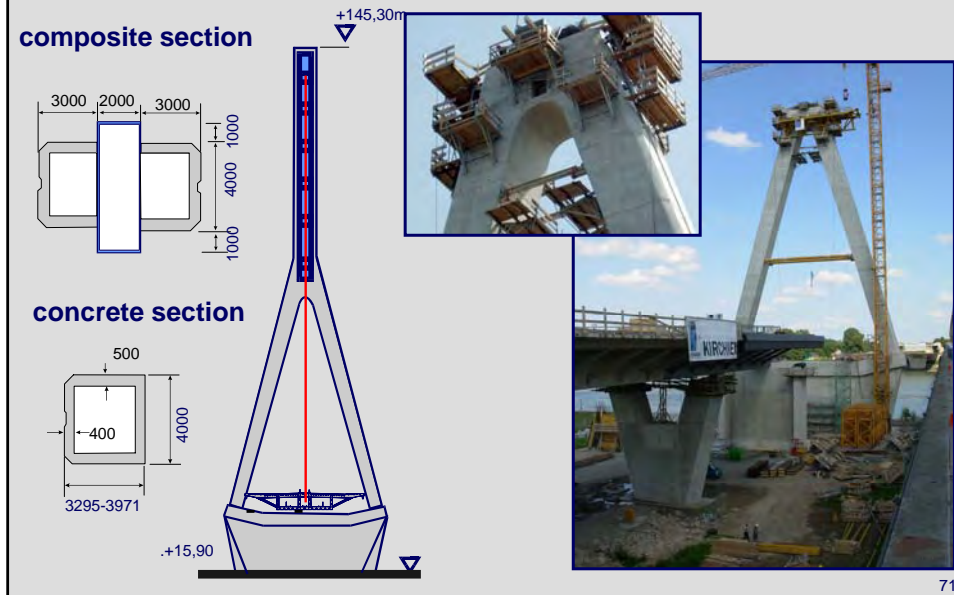
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concrete end cross girder

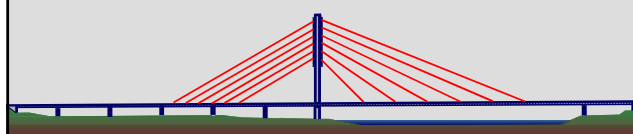


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Pylon in high strength concrete



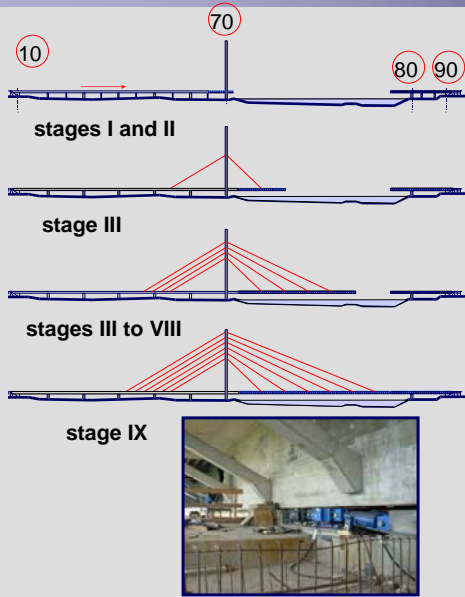
cables



Use of parallel strand cables of galvanized waxed and PE-coated strands instead of traditionally used fully locked coil ropes.

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erection stages



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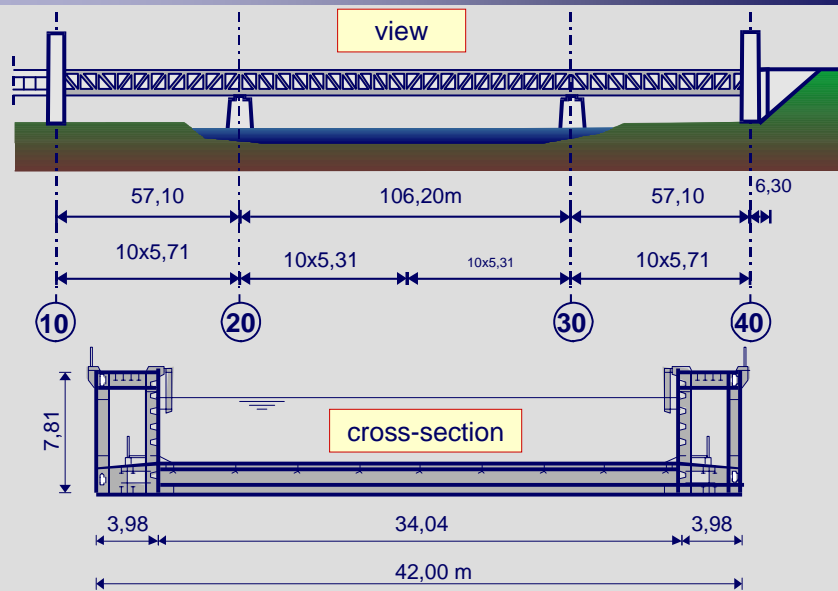
Canal bridges

canal bridge Lippe



4

Canal bridge Magdeburg

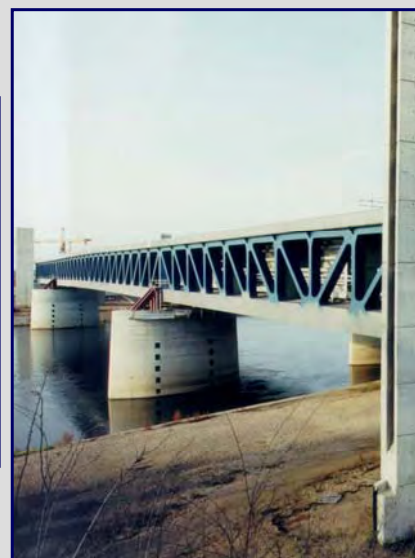


75

Canal bridge Magdeburg

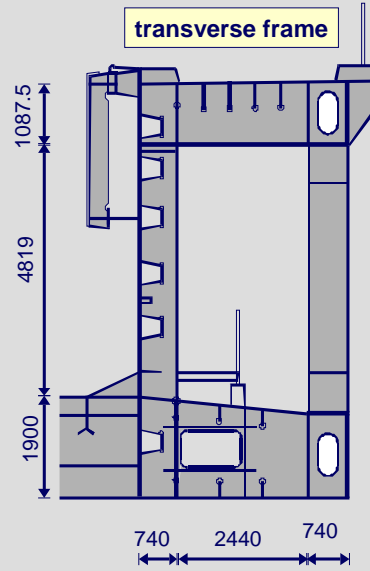


total length 228 m
span length 57,1 m – 106,2 m – 57,1 m
Structural steel 9.500 t



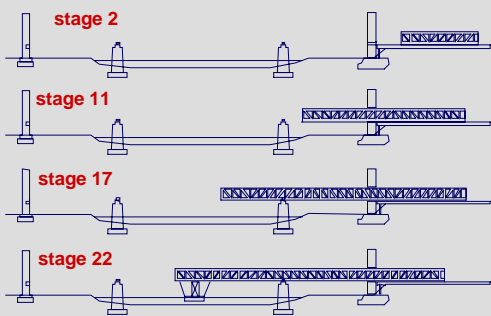
76

Canal bridge Magdeburg



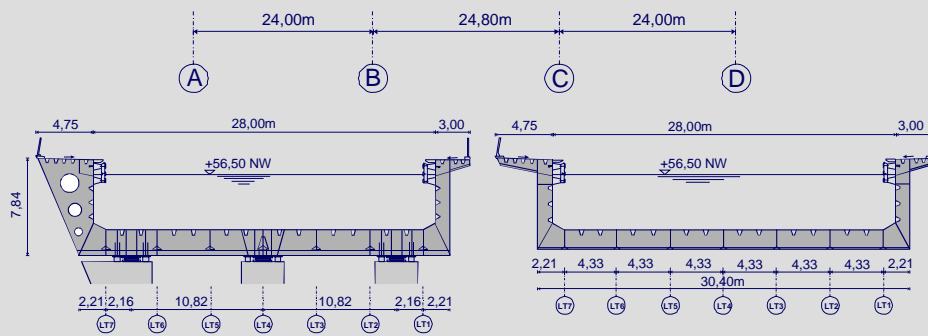
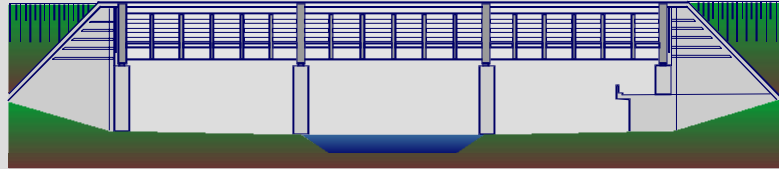
77

Launching of the bridge



78

Canal-bridge Lippe



79

Canal-bridge Lippe



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7th Japanese – German
Bridge Colloquium Osaka 2007



Thank you very much for your attention

